Northeastern States Materials Engineer's Association

Project-Level Analysis of Composite Pavements Using Ground Penetrating Radar

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Acknowledgements

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- The Need
- The Device
- Case Study No. 1 Dowels or I-Beams
- Case Study No. 2 Deleterious Material Under Composite Pavement
- Case Study No. 3 Voids Under Composite Pavement
- Recommendations on Practice
- On-Going Studies



The Need

- Ground-Penetrating RADAR
 has been around many
 decades
 - Improvements to use of 'off the shelf' equipment
 - Improvement to resolution at shallow depths
- Composite Pavements have unknown condition without costly excavation or





The Need

- Can we push the limits of resolution/technology to reduce need for open excavations
- Can off-the-shelf solutions provide in-the-field answers to subsurface uncertainties





- Ground-Penetrating RADAR
 - RAdio Detection And Ranging
- Generate wide-frequency pulse, interpret difference in waves as they return to the device.
- Applications in roadways back many decades, however technologic limits existed on data collection rate and frequency of antenna





- Case Study No. 1&3 Utilized
 - 1.6 GHz (No. 1 only)
 - 2.6 GHz
- Ground-coupled, analog
 antennas
- 2mm scan spacing/0.0174 nano-seconds



Antenna

Data Acquisition System



- Case Study No. 2 Utilized
 - 2.6 GHz
- Ground-coupled, analog, allin-one concrete scanning device





 Adapting a 3D Concrete Survey to Composite Pavements (Adding a layer to penetrate and interpret)



Photo Source: GSSI



- Lower frequencies → deeper penetration, but limited clarity at shallow depths
- Higher frequencies → shallower penetration, but higher clarity
- Ground-Coupled systems are slower to use
 - can't operate at highway speed
 - preserve energy lost at air/surface interface





Case Study No. 1: Differentiate Load Transfer Devices

- For a period of time, CT DOT permitted the use of i-beam style load transfer devices (LTDs) on Jointed Concrete pavements.
- It is unknown where these load transfer devices remain across the state, but the state desires to replace with modern dowels when encountered.





Case Study No. 1: Differentiate Load Transfer Devices

 Pseudo-Spectral Time Domain simulation performed to determine whether dowels vs. i-beams may be differentiated.



Lanbo Liu, Alexander Bernier, and James Mahoney, (2020), "Push the resolution limit: Can we differentiate the cross-section shape of dowel bars in the concrete with GPR?," SEG Global Meeting Abstracts : 180-183. https://doi.org/10.1190/gpr2020-049.1



Case Study No. 1: Differentiate Load Transfer Devices

- Success!
- Field-confirmed
 - length of LTDs
 - spacing of LTDs
 - structure layer thicknesses
- Hypothesize we did
 encounter i-beam LTDs
 at some locations





- 7-9 inches HMA over PCC in design
- 2.6 GHz all-in-one Ground-Coupled GPR unit
- Surface Distresses prompted investigation







TYPICAL SECTION NUMBERING KEY

- 1. APPLICATION OF GRADE
- 4" PMA S0.5 (2 EQUAL LIFTS) ON TOP OF 1 1/4" TO 2 3/3" HMA S0.375 CURB TO CURB 2.
- 3,5* TO 5* HMA 51 (2 LIFTS) ON TRAVEL LANES з,
- 4, 2" HMA S0.5 ON TOP OF 1 1/4" HMA S0.375
- APPROXIMATE LIMITS OF EXISTING 5. CONCRETE PAVEMENT
- APPROXIMATE LOCATION OF EXISTING BITUMINOUS CONCRETE PAVEMENT
- MILL TO TOP OF CONCRETE PAVEMENT
- RUBBILIZATION OF EXISTING CONCRETE PAVEMENT
- MILL SHOULDERS / CLIMBING LANES FLUSH ÷0. TO ADJACENT CONCRETE

12.

10. MATERIAL FOR TACK COAT

- TURF ESTABLISHMENT WITH 6" TOPSOIL (MAX.) IN AREAS OF EDGEDRAIN OUTLETS 3-CABLE GUIDE RAILING (I-BEAM POSTS) METAL BEAM RAIL (TYPE R-B 350, TYPE MD-B 350) AS REQUIRED
- 13. BITUMINOUS CONCRETE PARK CURBING
- REMOVE CURBING (REMOVAL OF B.C.L.C. WILL BE PAID FOR UNDER THE ITEM 14. "EARTH EXCAVATION")
- 15. 4" EDGEDRAIN

11.

- 16. 3" HMA SO.5 (1 LIFT)
- 17. MILL 3" MAX,
- 18. PROCESSED AGGREGATE
- 19. 3" HMA S0.5 (2 EQUAL LIFTS) ON SHOULDERS AND CLIMBING LANES





- Asphalt layer found to be thicker than original design
- 'Pasty' Effluent and light colored material indicative of a deleterious patch material







 In the field, wavy subgrade signals from GPR seemed to relate to presence of patch material



 Performed analysis of Traffic-Speed Survey Devices (ARAN + iPAVE) to identify potential other sites





- Air-Couple GPR identified location void to be fieldverified with Deflectometer Testing prior to repair
- Deflectometer indicated
 no repair necessary
- 2.6GHz Ground-Coupled brought in to see which NDT method it aligned with
- Attempted On-board 3D scanning software from Controller





- Laid out a 1-ft grid
- Longitudinal + Transverse







- Screen shots from field analysis: Panel 1
 - Seems like voids may exist
 - Unable to core/excavate to confirm
 - Asphalt layer seen to thicken (perhaps for a super elevation)

Joint

Not a Joint





- Screen shots from field
 analysis: Panel 2
 - No visible 'deformations' of signal across the panel
- Moisture plays a role in reading reflections/scans

SCHOOL OF ENGINEERING































Recommendations + Improvements

- Lessons Learned Moving Forward &
 Future Tasks
 - Longitudinal Scans Only for void detection
 - Run 0.5 ft interval
 - Run Normal + Cross-Polarized
 Scans to boost clarity in
 presence of welded wire
 - Build laboratory mock-ups of known composite conditions
 - Truck-Mounted Scanning for longer/faster Collection
 - Ground truthing dielectric for core/scan pairings in CT





Photo Source: GSSI

Current Studies: Pushing the limits

- Can we detect inter-asphalt layer differentials?
- Scans performed in the vicinity of a sand/skim layer
- Cores and test-pits performed as well
- Currently analyzing GPR data for possible identification
- Challenge: Sensitivity of the equipment to detect change in resistivity between different densities of material and accounting for noise of measurement





Questions?

Thank You!

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