



Ultra-High Performance Concrete for Steel Beam End Repairs

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Outline





Introduction to the Problem



America's Crumbling Infrastructure UCONN



ASCE Report Card





60,000 of the nation's bridges were STRUCTURALLY DEFICIENT in 2016



\$123 billion to catch up with the BACKLOG

of rehabilitation projects



Problem – Corrosion Damage

- 15% of structurally deficient bridges experience heavy corrosion damage
- \$8.3 billion is spent annually to repair or replace highway bridges with corrosion
- Girder bridges mainly suffer from corrosion at the girder ends
- The main cause of corrosion damage is deicing chemicals



Impact on the Structural Capacity









Current Repair Method



Overview of Proposed Repair Method





Repair Concept





Load Transfer Mechanism





Ultra High Performance Concrete

Definition of UHPC (FHWA 2011)

- Material consisting of optimized granular constituents
- Maximum W/C ratio of 0.25
- Compressive strength of larger than 22 ksi
- Enhanced durability via discontinuous pore structure
- □Self consolidating without vibration
- Constituents: cement, silica fume, silica powder, sand, water, admixtures, steel fibers

Component	Mass Fraction (%)	
Premix Powder	86.6	
Water	5.1	
Premia 150 (HRWR 1)	0.7	
Optima 100 (HRWR 2)	0.5	
Turbocast (Accelerator)	0.9	
Steel Fibers	6.2	

Ultra High Performance Concrete





Time [days]

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Phase 1: Large Scale Girder Tests





Tested three W21x55 Girders

- **1. Undamaged:** Baseline to measure capacity.
- **2. Damaged:** Web and flange reduction to simulate corrosion damage.
- **3. Repaired:** Same section reduction as Damaged, but with UHPC repair.



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Phase 1: Large-Scale Girder Tests

Undamaged Global Web Buckling



Damaged Localized Web Buckling



Repaired Flexural Yielding





Phase 1: Large-Scale Girder Tests





Phase 2: Development of Design Guide





Phase 2: Project Overview

Investigation of stud capacity in UHPC

Experimental study of full-scale plate girders

High fidelity finite element modeling and parametric studies

Development of design guide and standard details for field application



Behavior of shear studs in UHPC is different than in regular concrete.









Typical failure of push-out Specimens









Phase 2: Durability Testing



Prior to casting UHPC, the web of the steel section was submerged for 60 hours to achieve surface rust.



Phase 2: Durability Testing





Phase 2: Durability Testing







- 54"-Deep plate girders
- Test different repair geometries:
 - Full Height
 - Half Height
- Simulate Live Load







Damaged Girder

Half-Height Repair

Full-Height Repair









Repair Design Example





Pilot Bridge Repair Design

- Constructed in 1965
- □ Four simply supported spans
- Rolled steel multi-girder superstructure





Pilot Bridge Repair Design





Current Condition





Design Number of Shear Studs

		Demand (kip)	# of ½" Studs	# of % ″ Studs
Load Design	Live Load Only	116	9	6
	Strength I (AASHTO)	301	22	15
Capacity Design	Bearing	640	48	32
	Shear	568	42	28





Sample Repair Design





Fatigue Control

For studs embedded in UHPC (Cao 2017)

 $8 \log \Delta \tau + \log N = 22.1131$

ADT = 67,000 vehicles per day

N = 147 million cycles (50 year design life)

Allowable stress range (based on fatigue)= 7.9 ksi

Stress range demand under fatigue truck= 7.0 ksi



Summary and Conclusions



Summary and Conclusion

- A novel method has been developed to rehabilitate girders with corrosion damage by attaching UHPC panels to the girder using shear studs.
- The composite action of the UHPC repair introduced a secondary load path through the shear studs to transfer bearing and shear forces.
- The UHPC repair demonstrated the ability to restore bearing capacity lost due to corrosion over 5 times that of a damaged girder.
- This repair offers flexibility regarding location and number of studs to tackle complex geometries under various limit states



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Repair for Steel Bridge Girders with Corrosion Damage Utilizing UHPC

Thank you!

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What is the current common method for repairing deteriorated steel girder end regions? (single answer)

- a) Apply post-tensioning along length of girder tension flange,
- b) Weld or bolt new steel plates over deteriorated steel,
- c) Encase the end 10' of each deteriorated girder in conventional concrete,
- d) Selectively apply steel studs and UHPC in and near deteriorated regions.



What are some advantages of using UHPC to repair deteriorated steel girder end regions? (multiple answer)

- a) UHPC can easily flow into tight spaces,
- b) UHPC can carry higher tensile and compressive stresses than conventional concrete,
- c) UHPC costs less than conventional concrete,
- d) UHPC is far more durable than conventional concrete.



The UHPC girder end repair offers flexibility regarding location and number of studs to tackle complex geometries under various limit states? (True/False)



The full scale structural testing of the UHPC repair solution demonstrated that the lost bearing and shear capacity can often be recovered in a properly detailed UHPC repair? (True/False)