Maine Mall Road Porous Pavement

85th NESMEA Conference October 7, 2009 South Portland, Maine

Project location

- Maine Mall Road, South Portland
- Urban location
- Mixed commuter and commercial traffic



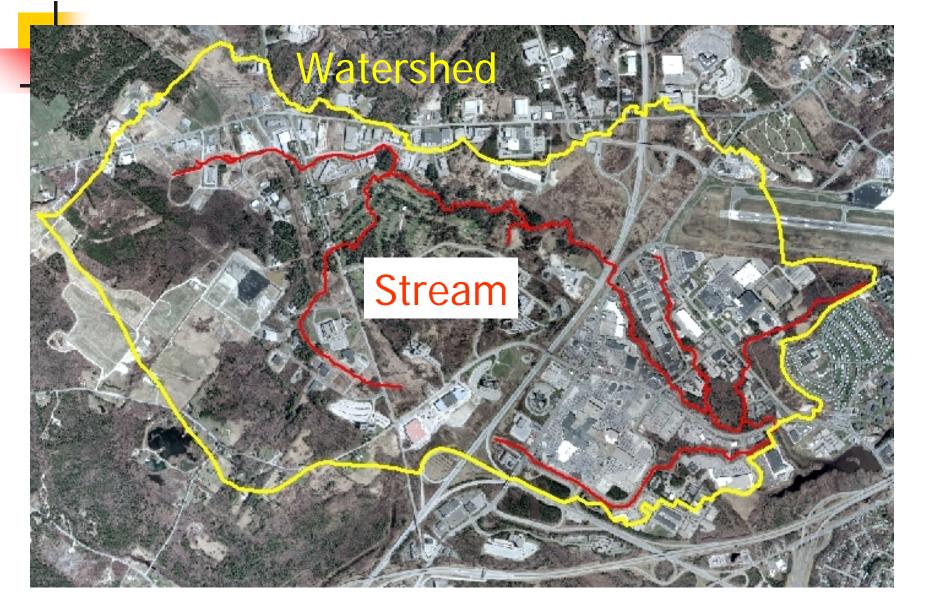
Project Information

- 0.35 miles in length
- AADT 16,750 vehicles/day
- ESALs 3,277,700 (20 years)
- Design Hourly Volume 2412
- Percent Heavy Trucks 5%
- Drains into South Branch of Long Creek

Why Porous Pavement?

- Long Creek is classified as an Urban Impaired Stream
- EPA Residual Designation Authority (2/09)
 - Require EXISTING development that contributes to the non attainment of water quality standards be regulated
 - Two affected waterways are Long Creek and Charles River in Boston

Long Creek Watershed



Long Creek Watershed

- Total watershed is 2200 acres; 640 acres (28 percent) is impervious
- All landowners with more than 1 acre of impervious surfaces - roofs, parking lots, roads - are regulated
- MaineDOT owns 64 acres of impervious surface within the Watershed

Permit options

- Individual Permit
 - Apply water quality practices on all 64 acres regardless of impact on stream
- General Permit
 - Participate in Long Creek Watershed Management Plan with other landowners and collectively treat priority areas

Why now?

- American Recovery and Reinvestment Act
 - 100 % Federal cost-sharing
- Satisfies part of MaineDOT Regulatory Requirements
- Opportunity to apply porous pavement to a urban highway

Porous pavement

- Proven technology
 - Over 20 years worldwide
 - Applications in colder climates
- Effective water quality treatment

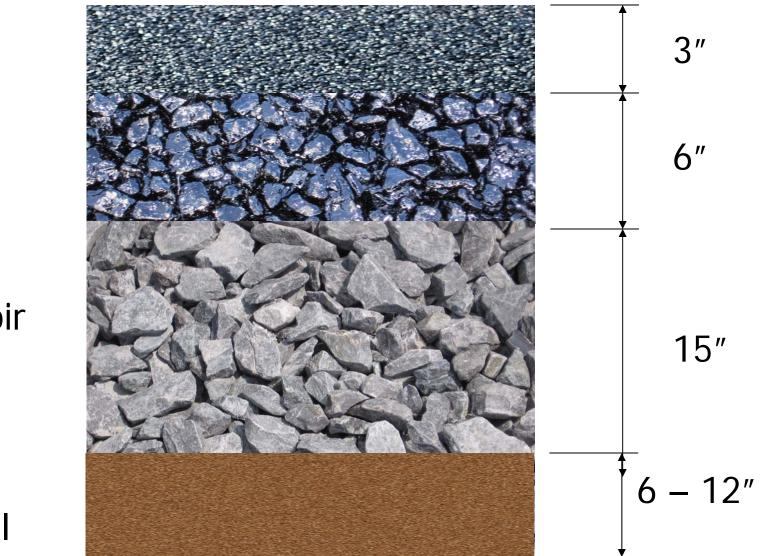
UNH Stormwater Center

 Other DOTs – Arizona, Oregon, North Carolina, Minnesota

Water quality advantage

- 3 key criteria
 - Detains Peak Stormwater Flow Rate
 - Pollutant Filtration
 - Temperature Mitigation

Pavement Section

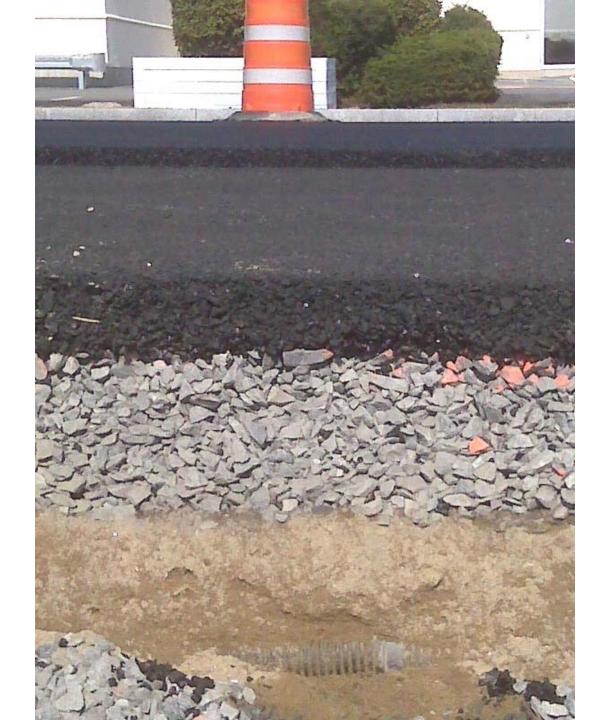


OGFC

ATPB

Reservoir Stone

Filter Material



Filter Material Layer

- Filters pollutants
- Help mitigate water temperature
- Includes 3 longitudinal runs of 6" perforated UD pipe, with laterals every 120' (each traffic direction)
- Filter material meets gradation for MaineDOT Type B underdrain sand





Reservoir Stone Layer

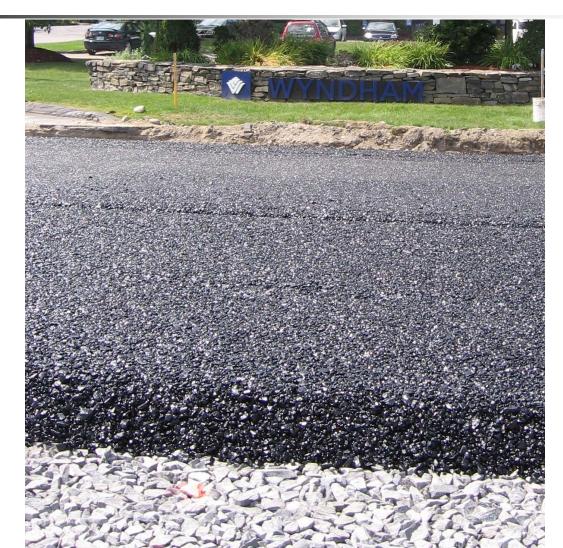


| AGGREGATE REQUIREMENTS | | |
|------------------------|-----------|--|
| 2-1/2" | 100 | |
| 2" | 95 – 100 | |
| 1" | 0 - 30 | |
| 3⁄4" | 0 – 5.0 | |
| L.A. Abrasion | 25.0 max. | |





Asphalt Treated Permeable Base



| GRADATION REQUIREMENTS | | |
|------------------------|--------------|--|
| 37.5 mm | 100 | |
| 25 mm | 95 – 100 | |
| 19 mm | 80 – 95 | |
| 12.5 mm | 35 – 70 | |
| 4.75 mm | 2 – 10 | |
| 2.36 mm | 0 – 5 | |
| 0.075 mm | 0 - 2.0 | |
| AGGREGATE QUALITIES | | |
| Micro-Deval | 18.0 maximum | |
| % Fractured | 85/80 | |
| Flat/Elongated | 10 | |

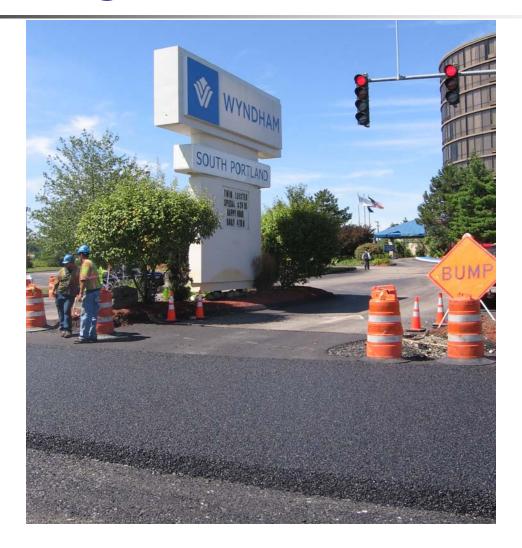
Mix requirements

- Binder : PG 76–28 with SBS polymer
- Minimum 2% binder
- Micro-Deval: 18.0 max. (composite blend)
- 95 percent coated particles (AASHTO T 195)
- 35 gyration design: looking for specimen that will be stable

JOB MIX FORMULA

| SIEVE SIZE | TARGET | SPEC RANGE |
|----------------|--------|-------------|
| 37.5 mm | 100 | 100 |
| 25 mm | 96 | 95 – 100 |
| 19 mm | 90 | 80 – 95 |
| 12.5 mm | 48 | 35 – 70 |
| 4.75 mm | 6 | 2 – 10 |
| 2.36 mm | 3 | 0 – 5 |
| 0.075 mm | 1.3 | 0 - 2.0 |
| Binder content | 2.0 | 2.0 minimum |

Open-graded Friction Course



| GRADATION REQUIREMENTS | | |
|------------------------|-----------|-----------|
| Sieve size | 12.5 mm | 9.5 mm |
| 19 mm | 100 | |
| 12.5 mm | 85 - 100 | 100 |
| 9.5 mm | 55 - 75 | 85 – 100 |
| 4.75 mm | 10 - 25 | 20 - 40 |
| 2.36 mm | 5 - 10 | 5 – 10 |
| 0.075 mm | 2.0 - 4.0 | 2.0 - 4.0 |
| | | |

| AGGREGATE QUALITIES | | |
|---------------------|--------------|--|
| Micro-Deval | 18.0 maximum | |
| % Fractured | 100/90 | |
| Flat/Elongated | 5 max. | |
| Sand equivalent | 50 | |
| FAA | 45 | |

| MIX DESIGN CRITERIA | | |
|------------------------------|------------------------------|--|
| Binder Grade | PG 76 - 28 w/SBS | |
| Binder content | 6.0% minimum | |
| Voids @ N _{des} | 20.0 percent | |
| VCA _{mix} | Less than VCA _{DRC} | |
| Draindown | 0.3% max. (AASHTO T 305) | |
| Cellulose fibers | 0.3 percent | |
| Gyrations @ N _{des} | 50 | |

JOB MIX FORMULA

| SIEVE SIZE | TARGET | SPEC RANGE |
|----------------|--------|--------------|
| 19 mm | 100 | 100 |
| 12.5 mm | 96 | 85 - 100 |
| 9.5 mm | 65 | 55 - 75 |
| 4.75 mm | 20 | 10 - 25 |
| 2.36 mm | 8 | 5 - 10 |
| 0.075 mm | 2.3 | 2.0 - 4.0 |
| Binder content | 6.0 | 6.0% minimum |

Construction specifications

- Track mounted paver
- Minimum air temperature:
 - 50°F ATPB
 - 60°F OGFC
- 12 ton static steel or 10 ton oscillatory roller minimum 3 passes
- Offsite test strip required
- No traffic for 24 hours

Construction specifications

- PGAB 64-28 Asphalt "Tack coat"
- All Construction Joints
 - Must be saw cut or milled unless paved in echelon
- Minimum placement temp of 290 deg. F

Limited construction activity on ATPB

Test strip

- Used to establish optimum laydown and compaction process
- Test strip supersedes specified equipment and techniques
- Completed strip provides opportunity to evaluate product

Something about white hats.... They make the crew nervous....



Maine Mall Road - Before

- Extremely flat in profile
- Project consisted of mill and overlay sections on each end of the project with porous pavement in the middle section
- Super-elevated turn lanes required carrying surface water to existing catchbasins several hundred feet away, or to the porous section
- Existing granite curb sections limited changes to roadway geometrics and drainage







- Track mounted paver caused minimal displacement to ballast stone surface
- Haul trucks displaced the ballast stone and required constant passes with a vibratory roller to keep stone locked
- Paver sonic trackers were erratic over the ballast stone, so were shut off after the first day of placement. Slope control was used to maintain cross slope







- Production and Placement Temperatures 290 340 deg. F.
- Breakdown roller temperature at approx. 200 210 deg. F to minimize lateral movement
- Mixture needed to "stiffen" enough to support compaction equipment



- ATPB placed at 7.5 inch and compacted to 6 inch finished depth
- ATPB mixture appeared to exhibit "tender" mix characteristics, with lateral movement if compacted at too high a temperature
- Placement was straightforward with few issues, with the exception of being extremely viscous (sticky)





There was a requirement to keep business access open during the ATPB placement operation

 Geotextile fabric and temporary mix was placed over the ballast stone and ATPB to maintain business access



- Surface and internal mixture temperatures monitored to determine compaction start
- 12 ton static roller was used as breakdown (approx. 200 – 210 deg.)
- 3-5 ton used as intermediate (approx 140 deg.)
- 10 ton static roller used as finish roller, with 3-5 to iron out any marks left behind





- Adjoining lanes placed the same day eliminated the requirement to mill or saw cut transverse or longitudinal construction joints
- Any construction joints left open were saw cut before continuing the placement of ATPB





- Like the ATPB, the surface and internal mixture temperatures monitored to determine compaction start
- 12 ton static roller was used as breakdown (approx. 180-210 deg. F)
- 3-5 ton used as intermediate (approx 140 deg. F)
- 10 ton static roller used as finish roller, with 3-5 and 1 ton rollers to iron out any marks left behind

Most in road structures raised and paved around with OGFC mixture

Some structures were set with concrete fill

Granite curb set and concrete fill placed along the gutter



- There was some damage to the ATPB surface due to the granite curb installation operation
- Damage was not considered severe enough to warrant remedial action, but illustrated the need to keep construction activity to a minimum



 RS-1 emulsified tack coat used around all structures and curbline rather than PGAB 64-28

 All concrete surfaces primed with approx. 0.05 gal/sq yard prior to OGFC placement





 OGFC was placed at a 3.75 inch depth, and compacted to 3 inch depth.

ATPB surface primed with approx.
 0.01 gal/sq yard prior to OGFC placement





- The mat was uniform, and free of any visual segregation
- The mixture did not exhibit the same "tender" characteristics as the ATPB
- Adjustments were made to the roller pattern and temperature range as the project progressed





- Joints were trimmed to a vertical edge while hot, to minimize trimming later.
- All construction joints were tacked with a heavy application of RS-1 emulsion
- Curb areas and utility structures took extra effort due to the coarse mixture













OGFC Placement

As with the ATPB, there was a requirement to keep business access open during the OGFC placement operation

This gave the crew the opportunity to "field test" the OGFC and ATPB layers

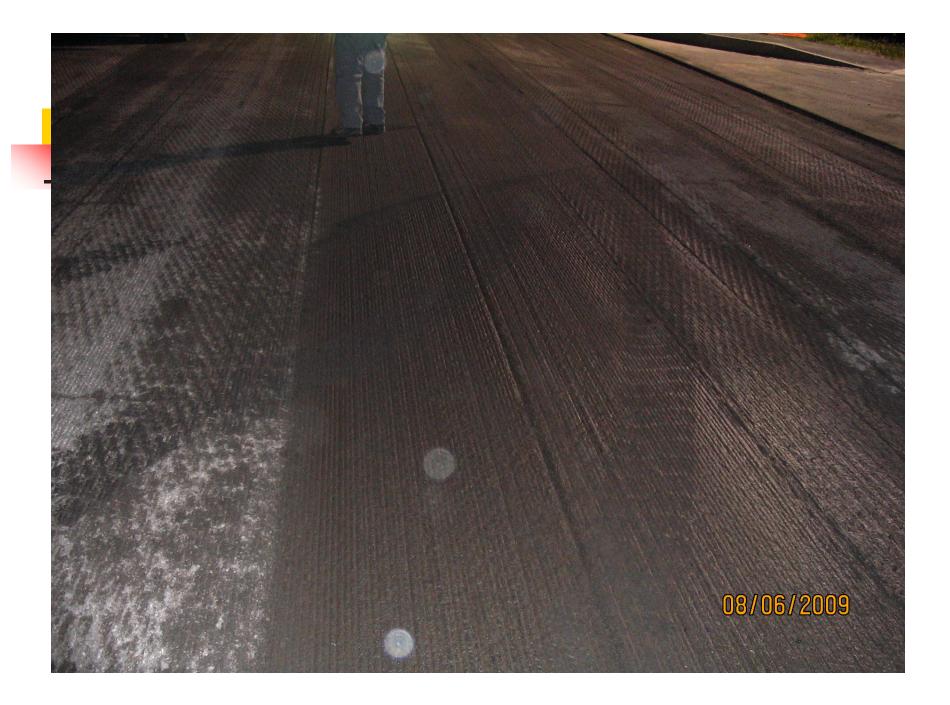




Mill and Overlay Sections 12.5mm w/ PGAB 76-28 SBS

- The project was reviewed for drainage issues, and elevations were taken prior to and after the project was milled.
- The project was milled at night. The old surface layer (s) varied in depth from 1.5 to 2.5 inches
- Areas of delamination were minimal.





Mill and Overlay Sections 12.5mm w/ PGAB 76-28 SBS

- A 9.5mm leveling course using PGAB 64-28 was placed over the milled surface to correct any surface irregularities and drainage concerns
- Structures were reset to the level course
- Due to the pavement removal depth being greater than estimated, the surface was changed to a 12.5mm PGAB 76-28 mixture, placed at a 1.5 inch depth



Overlay Sections 12.5mm w/ PGAB 76-28 SBS

- Production and Placement Temperatures
 290 350 deg. F.
- Compaction Temperatures approx. 290+ for breakdown roller
- Pneumatic roller left off until 190+/surface temp due to tire pick-up issues
- Finish rolled when Pneumatic was off







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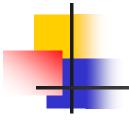
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South Portland - Maine Mall Road Porous Pavement Field Testing





Questions ?